### CITY OF RICHMOND, VIRGINIA

## DEPARTMENT OF PARKS, RECREATION AND COMMUNITY FACILITIES



### PUMP HOUSE PARKING LOT TRAIL

#### **INDEX OF SHEETS**

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Altra The Control of C

VICINITY MAP

1"=2000'

**OCTOBER 25, 2019** 

#### OWNER

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CONTACT: THERESA SIMMONS
EMAIL: THERESA.SIMMONS@RMTAONLINE.ORG

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CONTACT: JONET PREVOST-WHITE
EMAIL: JONET.PREVOST-WHITE@RICHMONDGOV.COM

DEVELOPER
CITY OF RICHMOND DPRCF
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RICHMOND, VA 23225
TELEPHONE: (804) 201-5914
CONTACT: MICHAEL BURTON
EMAIL: MICHAEL.BURTON@RICHMONDGOV.COM

CIVIL ENGINEER
TIMMONS GROUP

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RICHMOND, VA 23225
TELEPHONE: (804) 200-6495
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EMAIL: LEEANN.GUDORP@TIMMONS.COM

# LANDSCAPE ARCHITECT TIMMONS GROUP 1001 BOULDERS PARKWAY, SUITE 300 RICHMOND, VA 23225 TELEPHONE: (804) 200-6424 CONTACT: SCOTT WILEY, PLA EMAIL: SCOTT.WILEY@TIMMONS.COM

**STORMWATER DRAINAGE QUANTITIES:** 

**EROSION & SEDIMENT CONTROL QUANTITIES:** 

RICHMOND STORMWATER MANAGEMENT PERMIT

6" PERFORATED UNDERDRAIN

12" DRAIN BASIN

12" HDPE PIPE

**6" CLEANOUT** 

SILT FENCE

**DIVERSION** 

SAFETY FENCE

**INLET PROTECTION** 

**PERMITS REQUIRED:** 

DRAINAGE SWALE

LAND DISTURBANCE

Site Development | Residential | Infrastructure | Technology

YOUR VISION ACHIEVED THROUGH OURS.

VIRGINIA | NORTH CAROLINA | WEST VIRGINIA

THIS DRAWING PREPARED AT THE

CORPORATE OFFICE

1001 Boulders Parkway, Suite 300 | Richmond, VA 23225

TEL 804.200.6500 FAX 804.560.1016 www.timmons.com

1 EA

30 LF

90 LF

4 EA

390 LF

0.52 AC

300 LF

200 LF

2 EA

450 LF

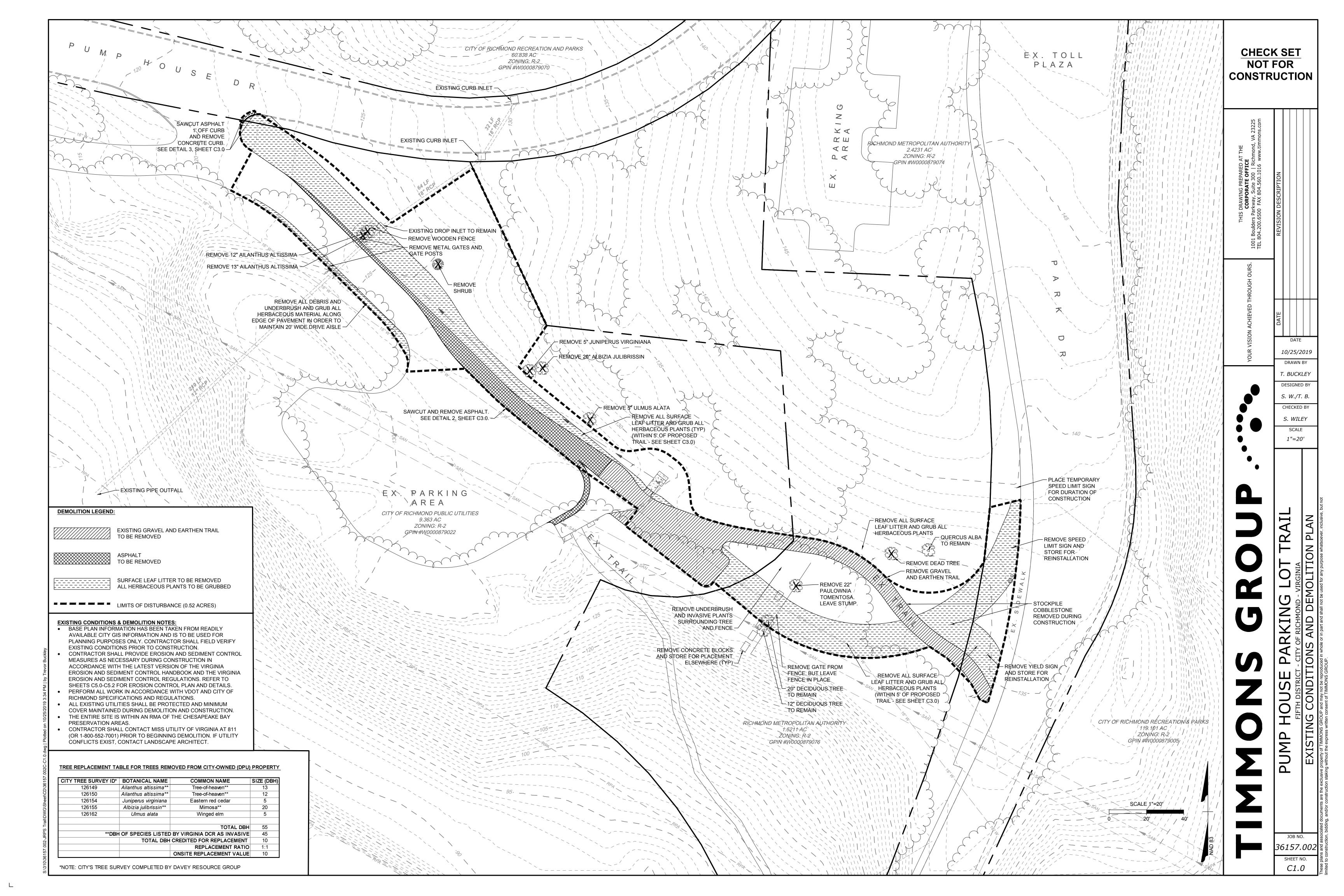
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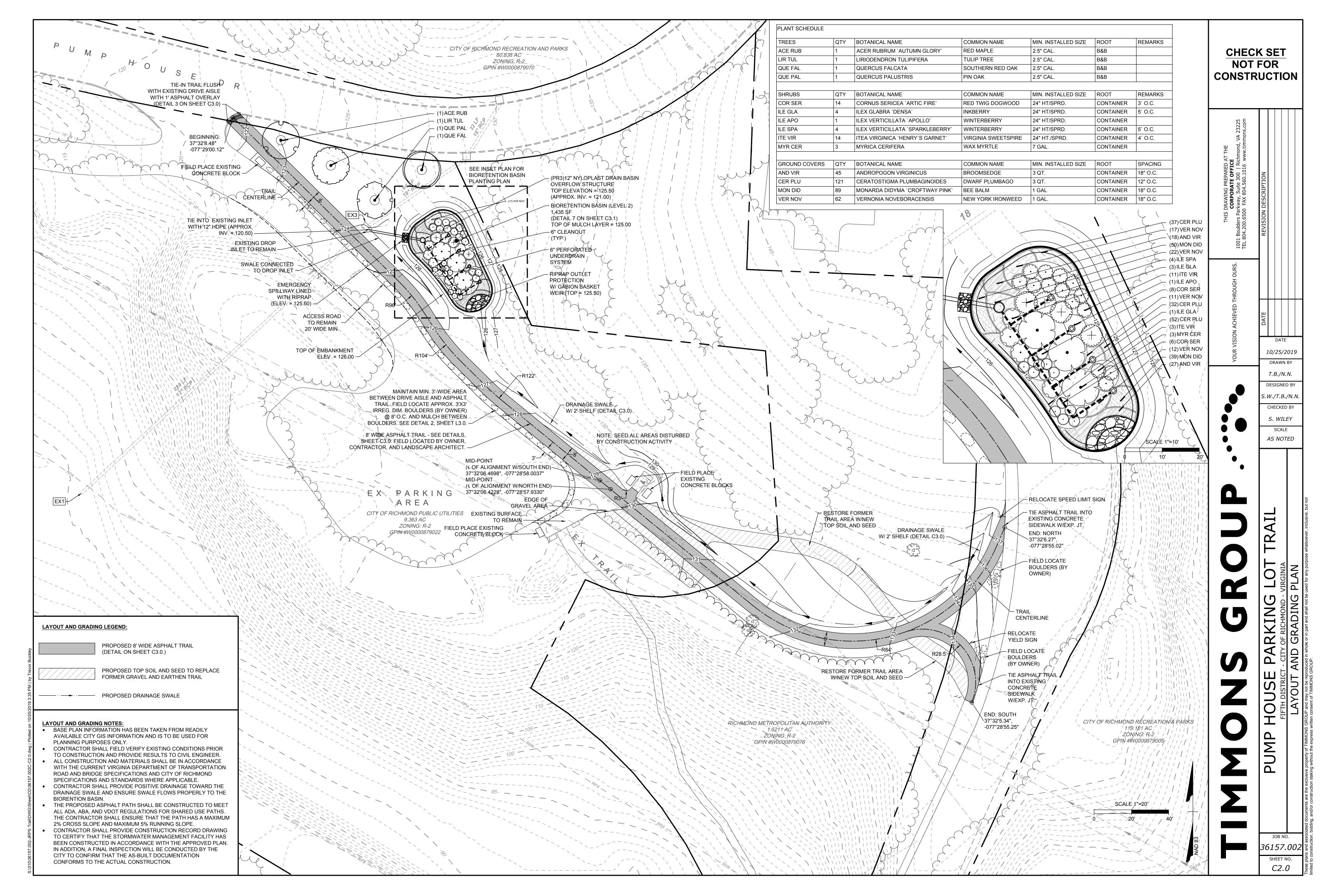
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-JRPS Trail\DWG\Sheet\CD\36157.002C-C0.0.dwg   Plotted on 10/25/2019 3:50 PM   by Trevor Buc	
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**REVISIONS** 

COMMENTS

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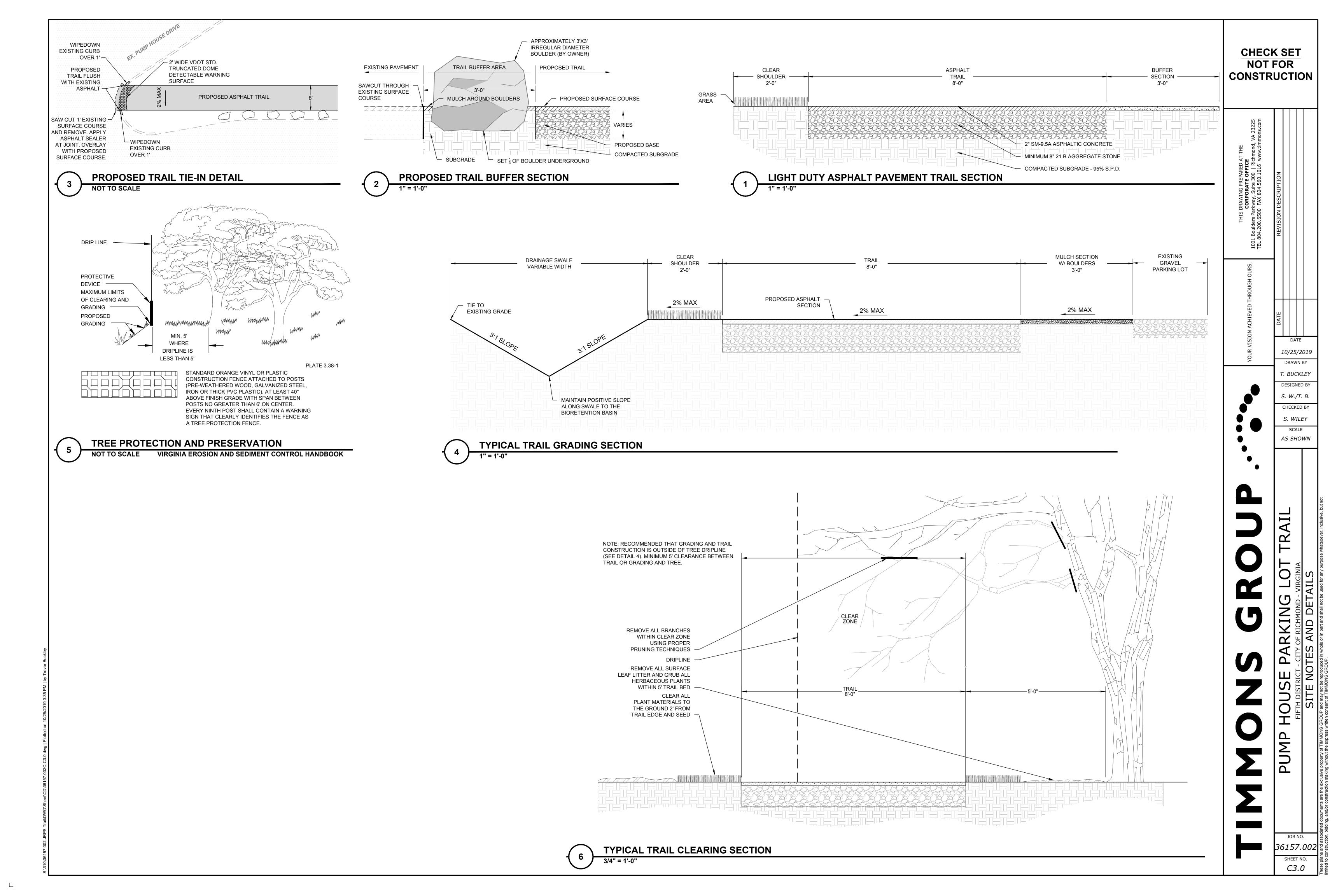


Table 9.7. Bioretention Material Specifications Material Specification Notes Filter Media to contain: The volume of filter media based on 110% 80% - 90% sand Filter Media of the plan volume, to account for settling 10%-20% soil fines Composition or compaction. 3%-5% organic matter Available P between L+ and M per DCR 2005 Nutrient Management Filter Media The media should be certified by the Testing Criteria. supplier. Use aged, shredded hardwood bark Lay a 2 to 3 inch layer on the surface of the Mulch Layer mulch or stable coarse compost. filter bed. Use river stone or pea gravel, coir and Lay a 2 to 3 inch layer of to suppress weed Alternative Surface Cover jute matting, or turf cover. Loamy sand or sandy loam texture, with less than 5% clay content, pH Top Soil 3 inch surface depth. corrected to between 6 and 7, and an For Turf Cover organic matter content of at least 2%. Apply only to the sides and directly above Use a non-woven geotextile fabric with the underdrain. For hotspots and certain Geotextile/Liner a flow rate of > 110 gal./min./sq. ft. karst sites only, use an appropriate liner on (e.g., Geotex 351 or equivalent) Lay a 2 to 4 inch layer of sand over a 2 inch layer of choker stone (typically #8 or #89 Choking Layer washed gravel), which is laid over the underdrain stone. Stone Jacket for inch stone should be double-washed 12 inches for the underdrain; Underdrain and clean and free of all fines (e.g., 12 to 18 inches for the stone storage layer, and/or Storage VDOT #57 stone). if needed Layer Use 6 inch rigid schedule 40 PVC pipe Lay the perforated pipe under the length of (or equivalent corrugated HDPE for the bioretention cell, and install non-Underdrains, micro-bioretention), with 3/8-inch perforated pipe as needed to connect with Cleanouts, and perforations at 6 inches on center; the storm drain system. Install T's and Y's Observation position each underdrain on a 1% or as needed, depending on the underdrain Wells 2% slope located nor more than 20 feet | configuration. Extend cleanout pipes to the surface with vented caps at the Ts and Ys. from the next pipe. Establish plant materials as specified in the Plant one tree per 250 square feet (15 landscaping plan and the recommended feet on-center, minimum 1 inch plant list. caliper). In general, plant spacing must be sufficient Shrubs a minimum of 30 inches high to ensure the plant material achieves 80% planted a minimum of 10 feet oncover in the proposed planting areas within Plant Materials a 3-year period. Plant ground cover plugs at 12 to 18 If seed mixes are used, they should be from inches on-center; Plant containera qualified supplier, should be appropriate grown plants at 18 to 24 inches onfor stormwater basin applications, and center, depending on the initial plant should consist of native species (unless the size and how large it will grow. seeding is to establish maintained turf).

#### STORM SEWER CAPACITY CALCULATIONS

STORM SEWER DESIGN COMPUTATIONS

**PROJECT**: PUMP HOUSE PARKING LOT TRAIL LOCATION: RICHMOND, VA

Designed by: NAN

COUNTY:

Checked by: LAG

Checked: LAG

STORM FREQUENCY 10 YR

UNITS ENGLISH

					DRAIN.	RUNOFF		CA		TOTAL	RAIN	RUNOFF	INVERT EL	EVATIONS	LENGTH	SLOPE	SIZE	SHAPE	Capacity	Friction	VEL	FLOW	
PIPE	FROM P	OINT	ТО РО	INT	AREA	COEFF.	INCRE-	ACCUM-	ADDTL	INLET	FALL	Q	UPPER	LOWER	of Pipe		(Dia. Or			Slope	Vn	TIME	
NO	REFERENCE	STA.	REFERENCE	STA.	"A"	"C"	MENT	ULATED	CA	TIME			END	END			Span/Rise)						REMARKS
					Acre					Minutes	In/Hr	CFS			Ft.	Ft./Ft.	In.		CFS	Ft./Ft.	Ft/Sec	Sec.	
	(1)		(2)		(3)	(4)	(5)	(6)		(7)	(8)	(9)	(10)	(11)	(12)	(13)	(14)		(15)		(16)	(17)	(18)
PR2	PR3	0+00.00	EX3	0+30.73	0.000	0.000	0.000	0.198	0.198	0.000	9.422	1.866	121.000	120.500	30.728	0.016	12.000	Circular	4.922	0.002	5.847	0.088	
EX2	EX3	0+30.73	EX1	2+39.82	0.000	0.000	0.000	0.198	0.000	0.088	9.365	1.854	120.500	95.500	209.093	0.120	12.000	Circular	12.315	0.003	11.314	0.308	

#### STORM SEWER HGL CALCULATIONS

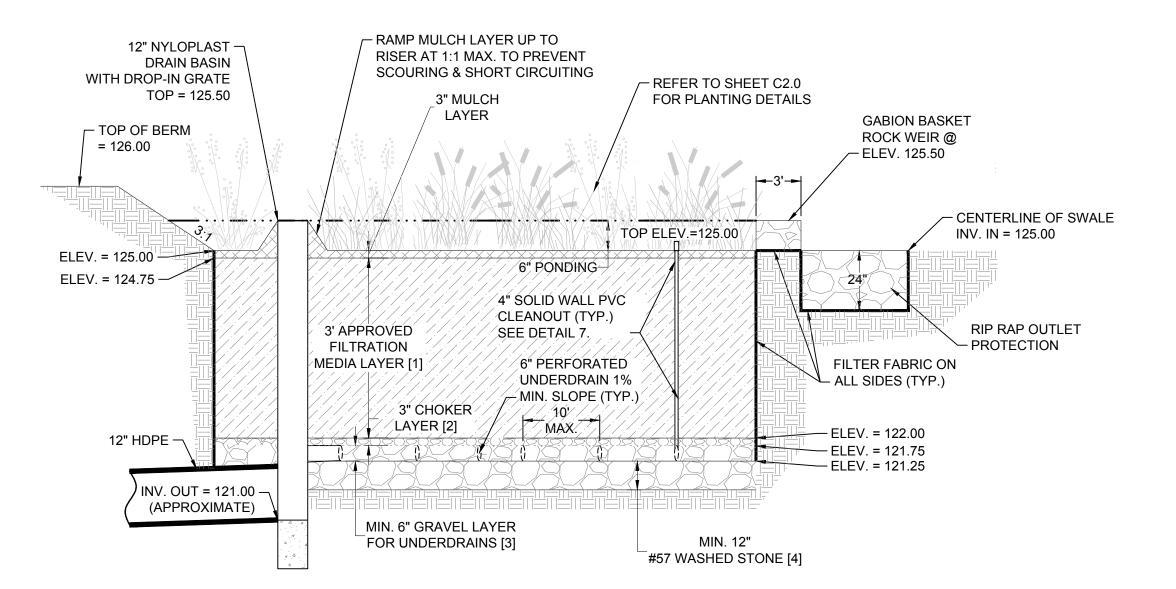
PUMP HOUSE PARKING LOT TRAIL HYDRAULIC GRADE LINE ANALYSIS DESIGNED BY: NAN

INCIDENCE PROBABILITY 10 Year

		OUTLET	DIA.	DESIGN	LENGTH	FRICTION	FRICTION					JL	INCTION LO	SS							Inlet		
INLET	INLET	WATER	PIPE	DISCH.	PIPE	SLOPE, Sfo	LOSS											1.3	0.5	FINAL	Water	Rim	Comments
OR	STATION	SURFACE	Do	Qo	Lo		Hf	Vo	Но	Qi	Vi	QiVi	Vi*2/2g	Hi	Angle	НΔ	Ht	Ht	Ht	Н	Surface	⊟ev	
JUNCTION		ELEV.	(ln)	(CFS)	(Ft)	(FT/FT)	(Ft)		(Ft)											(Ft)	⊟evation		
EX3	0+30.73	96.3	12	1.85	209.09	0.27%	0.57	11.31	0.5	1.87	5.85	10.91	0.53	0.19	55.86	0.3	0.98	0	FALSE	1.55	97.85	124	OK -26.15
PR3	0+00.00	121.3	12	1.87	30.73	0.23%	0.07	5.85	0.13	0	0	0	0	0	0	0	0.13	0	FALSE	0.2	121.5	125.5	OK -4

Table 9.8 Suggested Annual Maintenance Activities for Bioretention

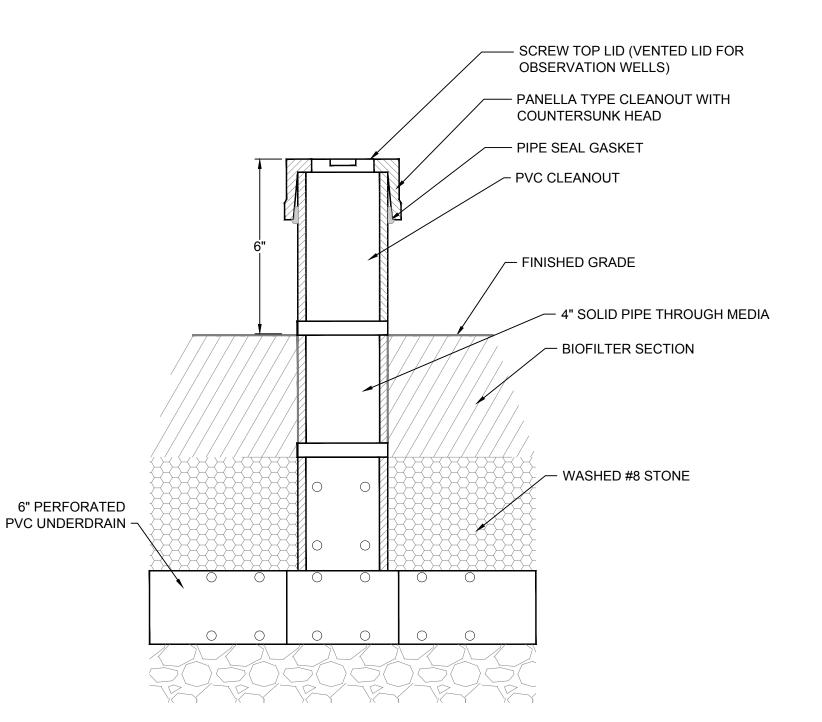
Table 9.6. Suggested Annual Maintenance Activities	S for bioretention
Maintenance Tasks	Frequency
<ul> <li>Mowing of grass filter strips and bioretention turf cover</li> </ul>	At least 4 times a year
<ul> <li>Spot weeding, erosion repair, trash removal, and mulch raking</li> </ul>	Twice during growing season
<ul> <li>Add reinforcement planting to maintain desired the vegetation density</li> <li>Remove invasive plants using recommended control methods</li> <li>Stabilize the contributing drainage area to prevent erosion</li> </ul>	As needed
<ul> <li>Spring inspection and cleanup</li> <li>Supplement mulch to maintain a 3 inch layer</li> <li>Prune trees and shrubs</li> </ul>	Annually
Remove sediment in pre-treatment cells and inflow points	Once every 2 to 3 years
Replace the mulch layer	Every 3 years



NOTE: ALL ELEVATIONS ARE APPROXIMATE BASED ON GIS INFORMATION AND ARE SUBJECT TO CHANGE DUE TO FIELD CONDITIONS.

- [1] 36" BIOFILTER MEDIA MIX (MUST HAVE PHOSPHOROUS INDEX, P-INDEX BETWEEN 10 30, OR BETWEEN 7 MG/KG & 21 MG/KG OF P)
- [2] 3" WASHED #8 STONE BRIDGE LAYER [3] 6" #57 WASHED STONE
- [4] 12" MIN. #57 WASHED STONE BELOW UNDERDRAIN INVERT. INCREASE STONE DEPTH AS NEEDED TO MAKE CONTACT W/ UNCOMPACTED NATIVE SOILS (APPROX. 1-2 FT).
- INSTALL EROSION AND SEDIMENT CONTROL MEASURES FOR THE SITE AND BASIN. GRADE SITE AROUND BIORETENTION BASIN AND EXCAVATE THE BIORETENTION BASIN TO WITHIN ONE FOOT OF FINISHED GRADE AT THE BOTTOM OF BASIN. DO
- NOT EXCAVATE TO FINISHED GRADES UNTIL SURROUNDING DRAINAGE AREAS HAVE BEEN STABILIZED. 3. COMPLETE CONSTRUCTION WITHIN BIORETENTION BASIN DRAINAGE AREA. STABILIZE BIORETENTION DRAINAGE AREA PRIOR TO BEGINNING CONSTRUCTION OF
- 4. REMOVE ANY ACCUMULATED SEDIMENT FROM BASIN AND EXCAVATE TO THE SUBGRADE DEPTH SPECIFIED ON THE BIORETENTION DETAIL. USE LIGHT WEIGHT
- TRACK EQUIPMENT WITH THE BASIN FOOTPRINT TO AVOID COMPACTION.
- DEEPLY TILL THE SUBGRADE WITH ROTARY TILLERS OR DISC HARROW TO PROVIDE AN AERATED, POROUS SURFACE.
- INSTALL #57 WASHED STONE AND UNDERDRAINS PER DETAIL. INSTALL STONE BRIDGE LAYER ABOVE THE #57 WASHED STONE. LIGHTLY COMPACT WITH A LANDSCAPING ROLLER.
- THE SOIL MEDIA MUST CONFORM WITH THE MATERIAL SPECIFICATIONS IN TABLE 9.7 OF THE DEQ BIORETENTION SPECIFICATION (SEE TABLE THIS SHEET). INSTALL BIORETENTION PLANTING MEDIA IN LIFTS NOT EXCEEDING 18" IN DEPTH, LIGHTLY COMPACT EACH LIFT BY TAMPING OR ROLLED WITH A LANDSCAPING
- ROLLER. COMPLY WITH THE ABOVE TESTING PROCEDURES AND SPECIFICATIONS.
- 10. PROTECT BIORETENTION BASIN FROM ALL VEHICULAR TRAFFIC UPON INSTALLATION.
- 11. COMPLETE INSTALLATION OF OUTLET PROTECTION.
- 12. INSTALL VEGETATION AND GROUND COVER PER THE LANDSCAPE PLANS.
- CONTRACTOR SHALL PROVIDE CONSTRUCTION RECORD DRAWING TO CERTIFY THAT THE STORMWATER MANAGEMENT FACILITY HAS BEEN CONSTRUCTED IN ACCORDANCE WITH THE APPROVED PLAN. IN ADDITION, A FINAL INSPECTION WILL BE CONDUCTED BY THE CITY TO CONFIRM THAT THE AS-BUILT DOCUMENTATION CONFORMS TO THE ACTUAL CONSTRUCTION.







**CHECK SET NOT FOR** CONSTRUCTION

DRAWING PREPARED AT 'CORPORATE OFFICE arkway, Suite 300 | Richm PAX 804.560 1012

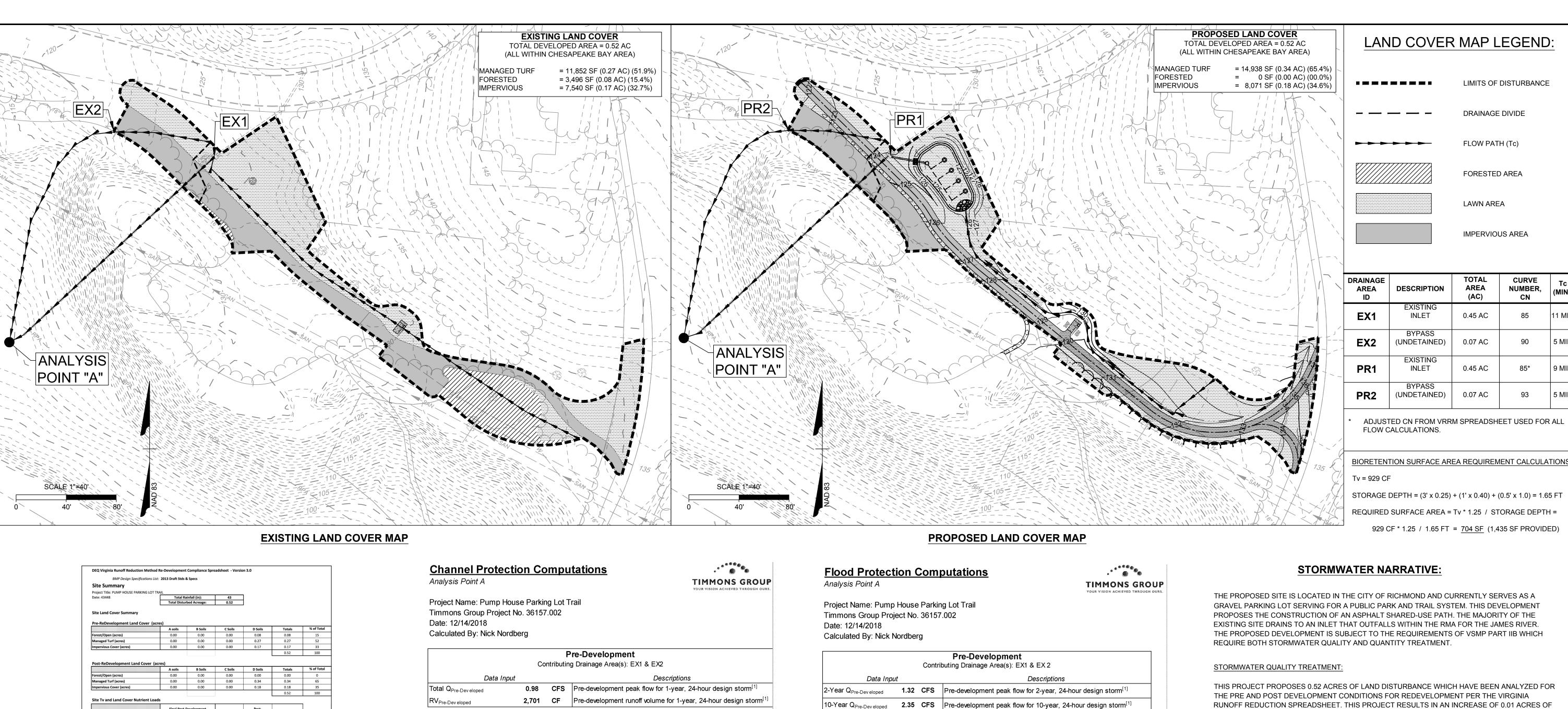
DATE

10/25/2019 DRAWN BY . NORDBERG **DESIGNED BY** 

I. NORDBERG CHECKED BY A. GUDORP

SCALE NONE

6157.002 SHEET NO. C3.1



Post-Development

Contributing Drainage Area(s): PR1 & PR2

Runoff reduced by BMPs

less than one (1) acre

Max. Permissible Q<sub>Post-Developed</sub>

1.12 cfs

Q<sub>Pre-Developed</sub>

0.98 cfs

WATER QUANTITY CALCULATIONS - CHANNEL PROTECTION

(from Energy Balance

Descriptions

True post-development runoff volume for 1-year, 24-hour design sto

Check

OK

Check

OK

**0.80 CFS** Post-development peak flow for 1-year, 24-hour design storm<sup>[1]</sup>

Per DEQ requirement for projects that disturb

Raw runoff volume for 1-year, 24-hour design storm<sup>[1]</sup>

Data Input

2,134 CF

\* Runoff reduction accounted for in flow values through adjusted curve number

[1] Flow values obtained from PondPak. Calculation packet to be included separately.

2,134

0.9

|Total Q<sub>Post-Dev eloped</sub>

RV Reduction

RV<sub>Post-Dev eloped</sub>

Improvement Factor

Unadjusted RV<sub>Post-Dev eloped</sub>

**Energy Balance Equation** 

Actual Q<sub>Post-Developed</sub>

0.80 cfs

Peak Flow Check

0.80 cfs

Q<sub>Post-Developed</sub>

#### Post-Development Contributing Drainage Area(s): PR1 & PR2 2-Year Q<sub>Post-Developed</sub> 1.16 CFS | Post-development peak flow for 2-year, 24-hour design storm<sup>[1]</sup>

10-Year Q<sub>Post-Developed</sub> 2.27 CFS | Post-development peak flow for 10-year, 24-hour design storm<sup>[1]</sup>

#### 2-Year Flood Protection Check

 $Q_{Post-Developed} \le Q_{Pre-Developed}$ 

Q <sub>Post-Dev</sub>	eloped	Q <sub>Pre-Developed</sub>	Check
1.16 cf	s ≤	1.32 cfs	ОК

#### **10-Year Flood Protection Check**

 $Q_{Post-Developed} \le Q_{Pre-Developed}$ 

Q <sub>Post-Develope</sub>	d	Q <sub>Pre-Developed</sub>	Check
2.27 cfs	≤	2.35 cfs	ок

#### \* Runoff reduction accounted for in flow values through adjusted curve number

[1] Flow values obtained from PondPak. Calculation packet to be included separately.

#### WATER QUANTITY CALCULATIONS - FLOOD PROTECTION

#### **STORMWATER NARRATIVE:**

THE PROPOSED SITE IS LOCATED IN THE CITY OF RICHMOND AND CURRENTLY SERVES AS A GRAVEL PARKING LOT SERVING FOR A PUBLIC PARK AND TRAIL SYSTEM. THIS DEVELOPMENT PROPOSES THE CONSTRUCTION OF AN ASPHALT SHARED-USE PATH. THE MAJORITY OF THE EXISTING SITE DRAINS TO AN INLET THAT OUTFALLS WITHIN THE RMA FOR THE JAMES RIVER. THE PROPOSED DEVELOPMENT IS SUBJECT TO THE REQUIREMENTS OF VSMP PART IIB WHICH REQUIRE BOTH STORMWATER QUALITY AND QUANTITY TREATMENT.

THIS PROJECT PROPOSES 0.52 ACRES OF LAND DISTURBANCE WHICH HAVE BEEN ANALYZED FOR THE PRE AND POST DEVELOPMENT CONDITIONS FOR REDEVELOPMENT PER THE VIRGINIA RUNOFF REDUCTION SPREADSHEET. THIS PROJECT RESULTS IN AN INCREASE OF 0.01 ACRES OF IMPERVIOUS AREA. THE RESULTING TP REMOVAL REQUIRED FOR THE PROPOSED DISTURBANCE IS **0.10 LB/YR**.

THE TOTAL PHOSPHORUS LOAD REDUCTION REQUIRED FOR THE DEVELOPMENT WILL BE MET THROUGH A PROPOSED BIORETENTION FACILITY. THERE WILL BE 0.45 ACRES DRAINING TO THE PROPOSED BMP INCLUDING 0.13 ACRES OF IMPERVIOUS AREA, WHICH RESULTS IN 0.42 LB/YR OF TP REMOVAL, EXCEEDING THE REQUIRED AMOUNT.

#### STORMWATER QUANTITY TREATMENT:

PROTECTION REQUIREMENTS AS PART OF IIB OF THE VIRGINIA STORMWATER MANAGEMENT REGULATIONS. ONLY THE DISTURBED AREA WILL BE ANALYZED AS PART OF THIS DEVELOPMENT THE DISTURBED AREA IS DIVIDED INTO TWO DRAINAGE AREAS: 0.45 ACRES WILL DRAIN TO THE PROPOSED BMP AND OUTFALL TO THE EXISTING STORM SYSTEM, AND 0.07 ACRES WILL BYPASS THE STORM SYSTEM UN-DETAINED. BOTH DRAINAGE AREAS WILL CONVERGE DOWNSTREAM AT POINT OF ANALYSIS "A".

THE 10-YEAR FLOW RATE IS USED TO PROVE FLOOD PROTECTION REQUIREMENTS ARE MET. AN ADJUSTED CURVE NUMBER IS USED TO CALCULATE POST-DEVELOPMENT FLOW RATES DUE TO THE RUNOFF REDUCTION PROVIDED THROUGH THE BIORETENTION BASIN. THE ALLOWABLE FLOW RATE FOR ANALYSIS POINT 1 IS 2.35 CFS AND THE POST-DEVELOPMENT FLOW IS 2.27 CFS.

THE PROPOSED DEVELOPMENT WILL MEET BOTH CHANNEL PROTECTION AND FLOOD

#### CHANNEL PROTECTION:

THE ENERGY BALANCE EQUATION IS USED TO PROVE CHANNEL PROTECTION REQUIREMENTS ARE MET FOR THE 1-YEAR STORM. AN ADJUSTED CURVE NUMBER IS USED TO CALCULATE POST-DEVELOPMENT FLOW RATES DUE TO THE RUNOFF REDUCTION PROVIDED THROUGH THE BIORETENTION BASIN. THE ALLOWABLE FLOW RATE FOR ANALYSIS POINT "A" IS 1.12 CFS AND THE POST-DEVELOPMENT FLOW IS **0.80 CFS**.

#### FLOOD PROTECTION:

#### **Runoff Volume and CN Calculations** 1-year storm 2-year storm 10-year storm 2.76 3.35 5.09 RV & CN Drainage Area A Drainage Area B Drainage Area C Drainage Area D Drainage Area E 1-year return period RV w RR (ws-in)

0.10 0.08 0.02

\*\* TARGET TP REDUCTION EXCEEDED BY 0.31 LB/YEAR \*\*

0.32 0.13 738.71 0.00 0.46 0.42

Managed Turf
Credit Area
(acres)

| BMP Treatment Volume (ft³) | TP Load from Upstream Practices (lbs) | TP Removed (lb/yr) | TP Remaining (lb/yr) | Downstream Treatment to Practice (lbs) | TP Removed (lb/yr) | TP Remaining (lb/y

Total TP Load Reduction Required (lb/yr)

Site Compliance Summary

**Drainage Area A Summary** 

6.b. Bioretention #2 or Micro-Bioretention

#2 (Spec #9)

otal Impervious Cover Treated (acres) 0.13 Total Turf Area Treated (acres)
Total TP Load Reduction Achieved in D.A.

**Land Cover Summary** 

Managed Turf (acres)

**BMP Selections** 

WATER QUALITY CALCULATIONS

RV w RR (ws-in)

#### **Stormwater Management Facility Data**

		Stormwater		Location Acres Treated By Facility			Pollutant Removal, lbs							
Stormwater Management Facility Type	Stormwater Management Description	Management Facility Structure Number	Latitude	Longitude	Impervious Acres	Pervious Acres	Total Acres	TP	TN	TSS	Runoff captured, acre-feet	HUC (6th order) Of Location Of Facility	Impaired Water Segment To Which Facility Discharges	Ownership Of Facility (Public/Private)
Bioretention	Bioretention level 2	n/a	37.535451	-77.482897	0.13	0.32	0.450	0.420	3.050	n/a	0.011	JM86	James River	Public

**CHECK SET NOT FOR** CONSTRUCTION

LIMITS OF DISTURBANCE

DRAINAGE DIVIDE

FLOW PATH (Tc)

FORESTED AREA

**IMPERVIOUS AREA** 

CURVE

NUMBER,

CN

9 MIN

LAWN AREA

**AREA** 

(AC)

0.07 AC

0.45 AC

0.07 AC

10/25/2019 DRAWN BY . NORDBERG

**DESIGNED BY** N. NORDBERG CHECKED BY

A. GUDORF 1'' = 40'

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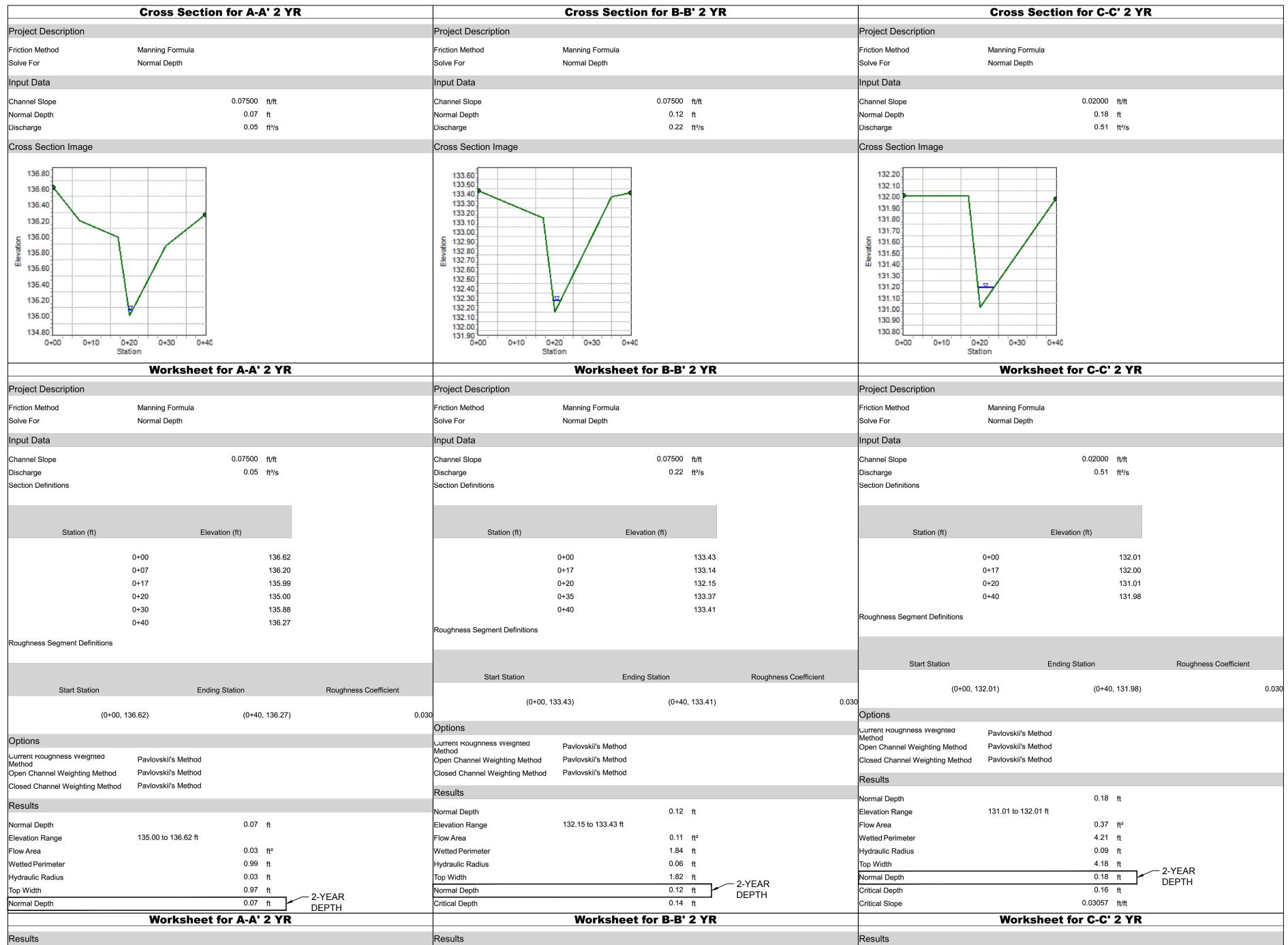
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86157.002 SHEET NO. C4.0

					GRASSED S	SWALE						
CROSS SECTION	IMPERVIOUS AREA (AC)	TURF AREA (AC)	DRAINAGE AREA (AC)	C-VALUE	CUMULATIVE DRAINAGE AREA (AC)	CUMULATIVE C-VALUE	CHANNEL SLOPE (FT/FT)	Tc (MIN)	2 YEAR S i (IN/HR)	TORM Q <sub>2</sub> (CFS)	10 YEAR S i (IN/HR)	TORM Q <sub>10</sub> (CFS)
A-A'	0.00	0.02	0.02	0.42	0.02	0.42	0.075	5.0	5.3	0.05	7.1	0.07
B-B'	0.01	0.07	0.08	0.43	0.10	0.42	0.075	5.2	5.3	0.22	7.0	0.29
C-C'	0.02	0.11	0.13	0.43	0.23	0.42	0.020	5.6	5.2	0.51	6.9	0.67
D-D'	0.01	0.18	0.19	0.38	0.43	0.40	0.020	5.9	5.1	0.87	6.8	1.16
E-E'	0.01	0.12	0.12	0.38	0.55	0.40	0.020	6.3	5.0	1.09	6.6	1.46
F-F'	0.01	0.14	0.14	0.38	0.69	0.40	0.020	6.6	4.9	1.35	6.5	1.79
G-G'	0.02	0.16	0.18	0.42	0.88	0.40	0.020	7.0	4.8	1.69	6.4	2.25
H-H'	0.15	0.11	0.26	0.66	1.14	0.46	0.020	7.4	4.7	2.48	6.3	3.31

	2 YEAR STORM	
В	D	Е
57.69	11.5	0.85

07.00		0.00
1	0 YEAR STORM	
В	D	Е
47.91	9.25	0.72



0.03247 ft/ft

2.04 ft/s

0.06 ft

0.18 ft

1.47

Supercritical

∠ 2-YEAR

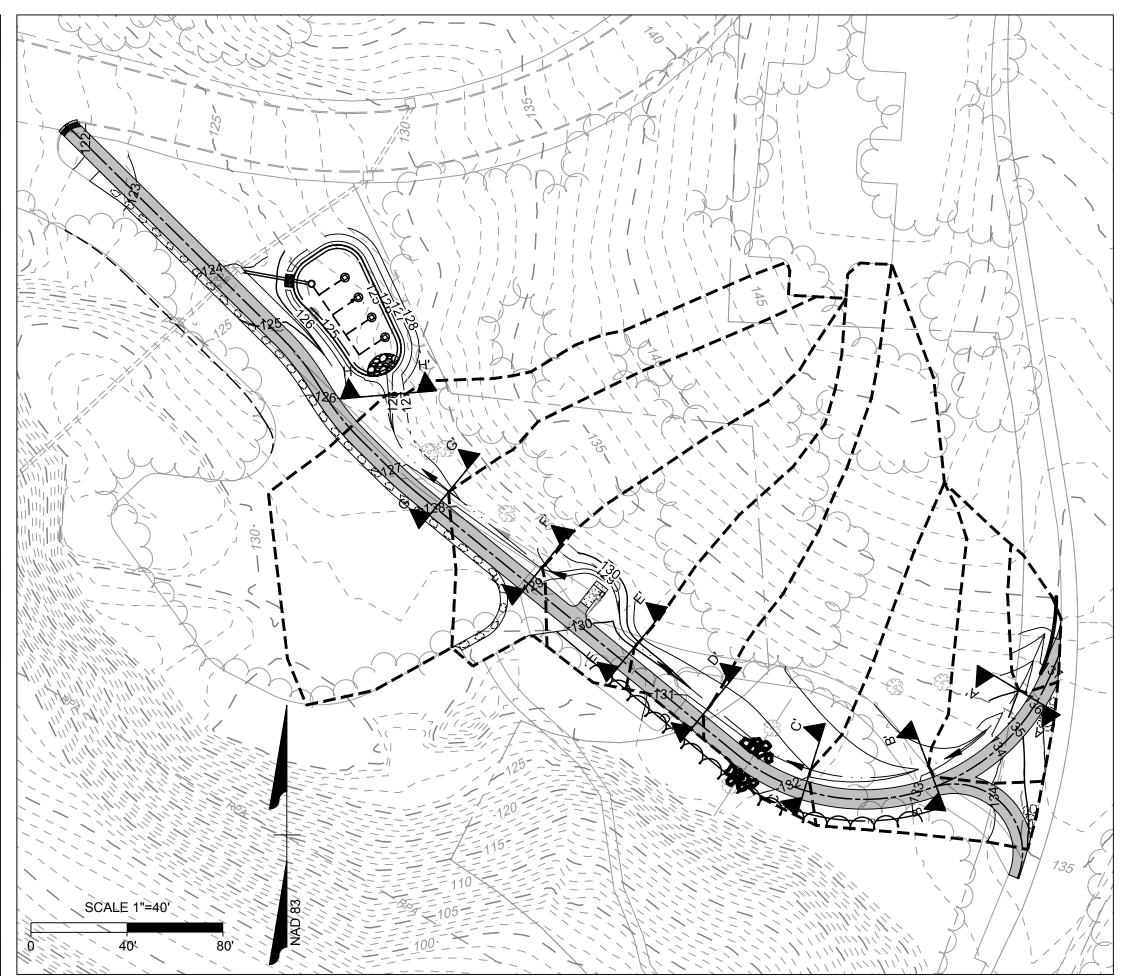
VELOCITY

Velocity Head

Specific Energy

roude Number

Flow Type



SWALE CROSS-SECTION DRAINAGE AREA MAP

1.38 ft/s 2-YEAR

0.03 ft

0.21 ft

0.82

Subcritical

VELOCITY

**CHECK SET** NOT FOR CONSTRUCTION

10/25/2019

N. NORDBERG DESIGNED BY N. NORDBERG CHECKED BY . A. GUDORP

1'' = 40'

TRAIL

**(** 

JOB NO. 36157.002 SHEET NO. C4.1

Critical Depth

Critical Slope

Velocity Head

Specific Energy

Flow Type

Froude Number

Velocity

0.08 ft

0.03 ft

0.10 ft

1.37

Supercritical

1.46 ft/s 2-YEAR

0.03917 ft/ft

Critical Slope

Velocity Head

Specific Energy

Froude Number

Flow Type

Velocity

**CHECK SET** NOT FOR CONSTRUCTION

10/25/2019 DRAWN BY N. NORDBERG N. NORDBERG CHECKED BY L. A. GUDORP

U

PUMP HOUSE PARKING L

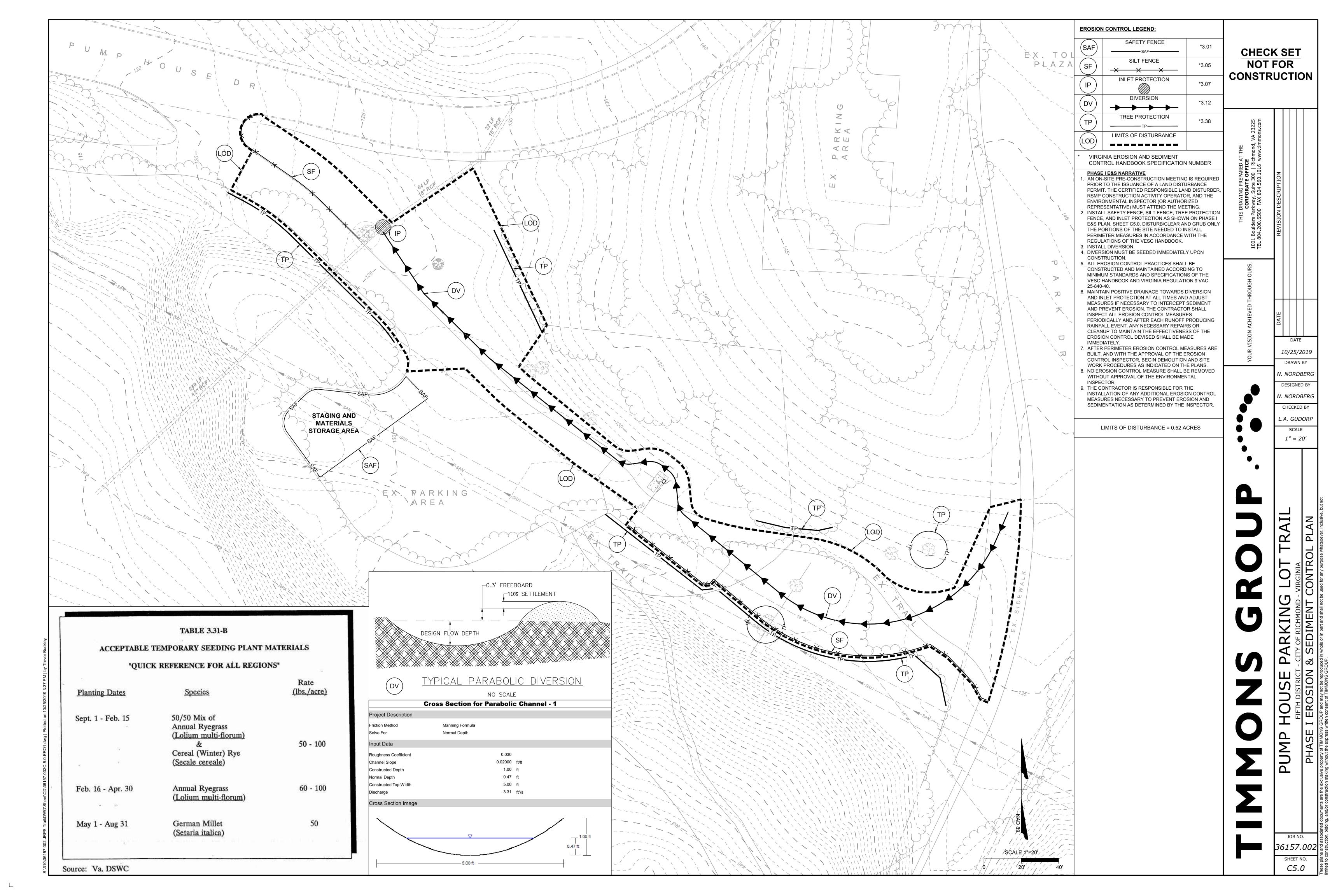
FIFTH DISTRICT - CITY OF RICHMOND - VIR

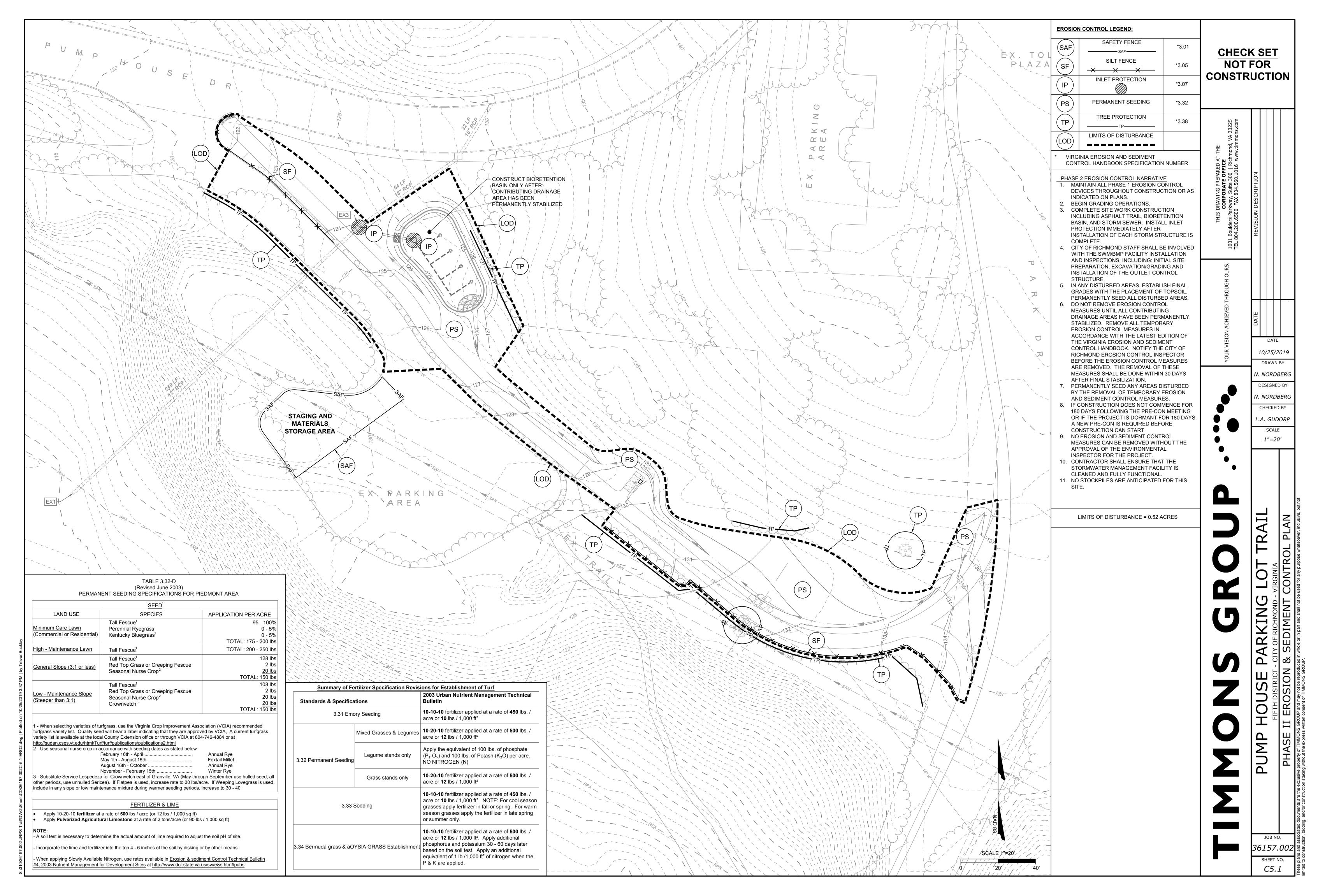
SWALE CROSS-SECTIONS AND CA

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36157.002 SHEET NO. **C4.2** 

	Cross Sectio	n for D-D' 2 YR		Cross Section	n for E-E' 2 YR		Cross Section fo	or F-F' 2 YR			Cross Section	for G-G' 2 YR		Cross Section	n for H-H' 2 YR	
Project Description			Project Description			Project Description				Project Description			Project Description			
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Source: Adapted from Installation of Straw and Fabric Filter Barriers for Sediment Control, Sherwood and Wyant

Plate 3.05-2

BLOCK & GRAVEL CURB INLET SEDIMENT FILTER

SPECIAL APPLICATION

THIS METHOD OF INLET PROTECTION IS APPLICABLE AT

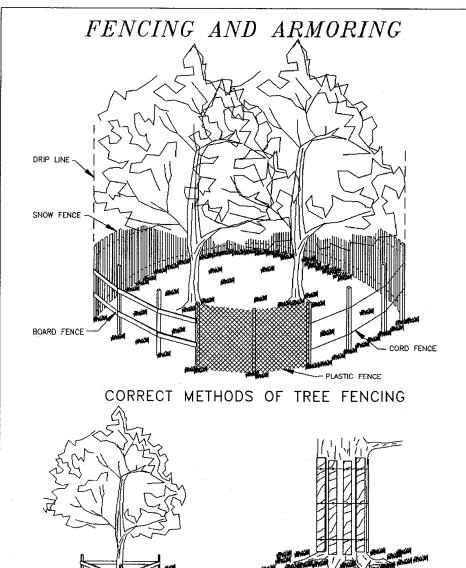
CURB INLETS WHERE AN OVERFLOW CAPABILITY IS NECESSARY

TO PREVENT EXCESSIVE PONDING IN FRONT OF THE STRUCTURE.

\* GRAVEL SHALL BE VDOT #3, #357 OR #5 COARSE AGGREGATE

Source: Va. DSWC

Plate 3.07-8



Source: Va. DSWC

TRIANGULAR BOARD FENCE

Plate 3.38-2

CORRECT TRUNK ARMORING

9VAC25-840-40. Minimum standards. (Effective 11/17/16)

A VESCP must be consistent with the following criteria, techniques and

1. Permanent or temporary soil stabilization shall be applied to denuded areas within seven days after final grade is reached on any portion of the site. Temporary soil stabilization shall be applied within seven days to denuded areas that may not be at final grade but will remain dormant for longer than 14 days. Permanent stabilization shall be applied to areas that are to be left dormant for more than one year.

2. During construction of the project, soil stock piles and borrow areas shall be stabilized or protected with sediment trapping measures. The applicant is responsible for the temporary protection and permanent stabilization of all soil stockpiles on site as well as borrow areas and soil intentionally transported from the project site.

3. A permanent vegetative cover shall be established on denuded areas not otherwise permanently stabilized. Permanent vegetation shall not be considered established until a ground cover is achieved that is uniform, mature enough to survive and will inhibit erosion.

4. Sediment basins and traps, perimeter dikes, sediment barriers and other measures intended to trap sediment shall be constructed as a first step in any land-disturbing activity and shall be made functional before upslope land disturbance takes place.

5. Stabilization measures shall be applied to earthen structures such as dams, dikes and diversions immediately after installation.

6. Sediment traps and sediment basins shall be designed and constructed based upon the total drainage area to be served by the trap or basin.

a. The minimum storage capacity of a sediment trap shall be 134 cubic yards per acre of drainage area and the trap shall only control drainage areas less than three acres.

b. Surface runoff from disturbed areas that is comprised of flow from drainage areas greater than or equal to three acres shall be controlled by a sediment basin. The minimum storage capacity of a sediment basin shall be 134 cubic yards per acre of drainage area. The outfall system shall, at a minimum, maintain the structural integrity of the basin during a 25-year storm of 24-hour duration. Runoff coefficients used in runoff calculations shall correspond to a bare earth condition or those conditions expected to exist while the sediment basin is utilized.

7. Cut and fill slopes shall be designed and constructed in a manner that will minimize erosion. Slopes that are found to be eroding excessively within one year of permanent stabilization shall be provided with additional slope stabilizing measures until the problem is corrected.

8. Concentrated runoff shall not flow down cut or fill slopes unless contained within an adequate temporary or permanent channel, flume or slope drain

9. Whenever water seeps from a slope face, adequate drainage or other protection shall be provided.

10. All storm sewer inlets that are made operable during construction shall be protected so that sediment-laden water cannot enter the conveyance system without first being filtered or otherwise treated to remove sediment.

11. Before newly constructed stormwater conveyance channels or pipes are made operational, adequate outlet protection and any required temporary or permanent channel lining shall be installed in both the conveyance channel and receiving channel.

12. When work in a live watercourse is performed, precautions shall be taken to minimize encroachment, control sediment transport and stabilize the work area to the greatest extent possible during construction. Nonerodible material shall be used for the construction of causeways and cofferdams. Earthen fill may be used for these structures if armored by nonerodible cover materials.

13. When a live watercourse must be crossed by construction vehicles more than twice in any six-month period, a temporary vehicular stream crossing constructed of nonerodible material shall be provided.

14. All applicable federal, state and local requirements pertaining to working in or crossing live watercourses shall be met.

15. The bed and banks of a watercourse shall be stabilized immediately after work in the watercourse is completed.

16. Underground utility lines shall be installed in accordance with the following standards in addition to other applicable criteria:

a. No more than 500 linear feet of trench may be opened at one time.

b. Excavated material shall be placed on the uphill side of trenches.

c. Effluent from dewatering operations shall be filtered or passed through an approved sediment trapping device, or both, and discharged in a manner that does not adversely affect flowing streams or off-site property.

d. Material used for backfilling trenches shall be properly compacted in order to minimize erosion and promote stabilization.

e. Restabilization shall be accomplished in accordance with this chapter.

f. Applicable safety requirements shall be complied with.

17. Where construction vehicle access routes intersect paved or public roads, provisions shall be made to minimize the transport of sediment by vehicular tracking onto the paved surface. Where sediment is transported onto a payed or public road surface, the road surface shall be cleaned thoroughly at the end of each day. Sediment shall be removed from the roads by shoveling or sweeping and transported to a sediment control disposal area. Street washing shall be allowed only after sediment is removed in this manner. This provision shall apply to individual development lots as well as to larger land-disturbing activities.

18. All temporary erosion and sediment control measures shall be removed within 30 days after final site stabilization or after the temporary measures are no longer needed, unless otherwise authorized by the VESCP authority. Trapped sediment and the disturbed soil areas resulting from the disposition of temporary measures shall be permanently stabilized to prevent further erosion and sedimentation.

19. Properties and waterways downstream from development sites shall be protected from sediment deposition, erosion and damage due to increases in volume, velocity and peak flow rate of stormwater runoff for the stated frequency storm of 24-hour duration in accordance with the following standards and criteria. Stream restoration and relocation projects that incorporate natural channel design concepts are not man-made channels and shall be exempt from any flow rate capacity and velocity requirements for natural or man-made channels:

a. Concentrated stormwater runoff leaving a development site shall be discharged directly into an adequate natural or man-made receiving channel, pipe or storm sewer system. For those sites where runoff is discharged into a pipe or pipe system, downstream stability analyses at the outfall of the pipe or pipe system shall be performed.

b. Adequacy of all channels and pipes shall be verified in the following

(1) The applicant shall demonstrate that the total drainage area to the point of analysis within the channel is 100 times greater than the contributing drainage area of the project in question; or

(2) (a) Natural channels shall be analyzed by the use of a two-year storm to verify that stormwater will not overtop channel banks nor cause erosion of channel bed or banks.

(b) All previously constructed man-made channels shall be analyzed by the use of a 10-year storm to verify that stormwater will not overtop its banks and by the use of a two-year storm to demonstrate that stormwater will not cause erosion of channel bed or banks; and

(c) Pipes and storm sewer systems shall be analyzed by the use of a 10-year storm to verify that stormwater will be contained within the pipe or system.

c. If existing natural receiving channels or previously constructed man-made channels or pipes are not adequate, the applicant shall:

(1) Improve the channels to a condition where a 10-year storm will not overtop the banks and a two-year storm will not cause erosion to the channel, the bed, or the banks; or

(2) Improve the pipe or pipe system to a condition where the 10-year storm is contained within the appurtenances:

(3) Develop a site design that will not cause the pre-development peak runoff rate from a two-year storm to increase when runoff outfalls into a natural channel or will not cause the pre-development peak runoff rate from a 10year storm to increase when runoff outfalls into a man-made channel; or

(4) Provide a combination of channel improvement, stormwater detention or other measures which is satisfactory to the VESCP authority to prevent downstream erosion.

d. The applicant shall provide evidence of permission to make the improvements.

e. All hydrologic analyses shall be based on the existing watershed characteristics and the ultimate development condition of the subject project.

f. If the applicant chooses an option that includes stormwater detention, he shall obtain approval from the VESCP of a plan for maintenance of the detention facilities. The plan shall set forth the maintenance requirements of the facility and the person responsible for performing the maintenance.

g. Outfall from a detention facility shall be discharged to a receiving channel, and energy dissipators shall be placed at the outfall of all detention facilities as necessary to provide a stabilized transition from the facility to the receiving channel.

h. All on-site channels must be verified to be adequate.

i. Increased volumes of sheet flows that may cause erosion or sedimentation on adjacent property shall be diverted to a stable outlet, adequate channel, pipe or pipe system, or to a detention facility.

j. In applying these stormwater management criteria, individual lots or parcels in a residential, commercial or industrial development shall not be considered to be separate development projects. Instead, the development, as a whole, shall be considered to be a single development project. Hydrologic parameters that reflect the ultimate development condition shall be used in all engineering calculations.

k. All measures used to protect properties and waterways shall be employed in a manner which minimizes impacts on the physical, chemical and biological integrity of rivers, streams and other waters of the state.

I. Any plan approved prior to July 1, 2014, that provides for stormwater management that addresses any flow rate capacity and velocity requirements for natural or man-made channels shall satisfy the flow rate capacity and velocity requirements for natural or man-made channels if the practices are designed to (i) detain the water quality volume and to release it over 48 hours; (ii) detain and release over a 24-hour period the expected rainfall resulting from the one year, 24-hour storm; and (iii) reduce the allowable peak flow rate resulting from the 1.5, 2, and 10-year, 24-hour storms to a level that is less than or equal to the peak flow rate from the site assuming it was in a good forested condition, achieved through multiplication of the forested peak flow rate by a reduction factor that is equal to the runoff volume from the site when it was in a good forested condition divided by the runoff volume from the site in its proposed condition, and shall be exempt from any flow rate capacity and velocity requirements for natural or manmade channels as defined in any regulations promulgated pursuant to § 62.1-44.15:54 or 62.1-44.15:65 of the Act.

m. For plans approved on and after July 1, 2014, the flow rate capacity and velocity requirements of § 62.1-44.15:52 A of the Act and this subsection shall be satisfied by compliance with water quantity requirements in the Stormwater Management Act (§ 62.1-44.15:24 et seq. of the Code of Virginia) and attendant regulations, unless such land-disturbing activities (i) are in accordance with provisions for time limits on applicability of approved design criteria in <u>9VAC25-870-47</u> or grandfathering in <u>9VAC25-870-48</u> of the Virginia Stormwater Management Program (VSMP) Regulation, in which case the flow rate capacity and velocity requirements of § 62.1-44.15:52 A of the Act shall apply, or (ii) are exempt pursuant to § 62.1-44.15:34 C 7 of the

n. Compliance with the water quantity minimum standards set out in 9VAC25-870-66 of the Virginia Stormwater Management Program (VSMP) Regulation shall be deemed to satisfy the requirements of this subdivision

Statutory Authority

§ 62.1-44.15:52 of the Code of Virginia

Historical Notes

Former 4VAC50-30-40, derived from VR625-02-00 § 4; eff. September 13, 1990; amended, Virginia Register Volume 11, Issue 11, eff. March 22, 1995; Volume 29, Issue 4, eff. November 21, 2012; amended and renumbered, Virginia Register Volume 30, Issue 2, eff. October 23, 2013; amended, Virginia Register Volume 31, Issue 24, eff. August 26, 2015; Volume 33, Issue 4, eff. November 17, 2016.

#### **Standard E&S Notes**

- 1. Permanent or temporary soil stabilization shall be applied to denuded areas within seven days after final grade is reached on any portion of the site. Temporary soil stabilization shall be applied within seven days to denuded areas that may not be at final grade but will remain, dormant (undisturbed) for longer than 14 days. Permanent stabilization shall be applied to areas that are to be left dormant for
- 2. Excess excavation disposed of off the site shall be disposed of in accordance with the Virginia Erosion and Sediment Control Handbook.
- 3. Erosion and Sediment Controls shall be installed in accordance with Virginia Erosion and Sediment Control Handbook and shall be placed prior to or as the first
- step of the land disturbing activity. 4. Erosion and Sediment Controls shall be maintained so that the sediment carrying runoff from the site will not enter storm drainage facilities.
- 5. Erosion and Sediment Controls shall be maintained until the disturbed area is
- 6. Properties adjoining the site shall be kept clean of mud or silt carried from the site by vehicular traffic or runoff.
- 7. The disposal of waste materials removed from erosion and sediment control facilities and the disposal of these facilities shall be in accordance with the Virginia Erosion and Sediment Control Handbook.
- 8. Stabilization measures shall be applied to earthen structures such as dams, dikes and diversions immediately after installation.
- 9. During construction of the project, soil stockpiles shall be stabilized or protected with sediment trapping measures. The applicant is responsible for the temporary protection and permanent stabilization of all soil stockpiles on site as well as soil intentionally transported from the project site.

TABLE 6-1 (Virginia Erosion and Sediment Control Handbook)

#### GENERAL EROSION AND SEDIMENT CONTROL NOTES

- ES-1: Unless otherwise indicated, all vegetative and structural erosion and sediment control practices will be constructed and maintained according to minimum standards and specifications of the Virginia Erosion and Sediment Control Handbook and the Virginia Erosion and Sediment Control Regulations 9VAC25-840.
- The plan approving authority must be notified one week prior to the pre-construction conference, one week prior to the commencement of land disturbing activity, and one week prior to the final inspection.
- ES-3: All erosion and sediment control measures are to be placed prior to or as the first step
- ES-4: A copy of the approved erosion and sediment control plan shall be maintained on the site at all times.
- ES-5: Prior to commencing land disturbing activities in areas other than indicated on these plans (including, but not limited to, off-site borrow or waste areas), the contractor shall submit a supplementary erosion control plan to the owner for review and approval by
- ES-6: The contractor is responsible for installation of any additional erosion control measures necessary to prevent erosion and sedimentation as determined by the plan approving authority.
- ES-7: All disturbed areas are to drain to approved sediment control measures at all times during land disturbing activities and during site development until final stabilization is achieved.
- ES-8: During dewatering operations, water will be pumped into an approved filtering device.
- ES-9: The contractor shall inspect all erosion control measures periodically and after each runoff-producing rainfall event. Any necessary repairs or cleanup to maintain the effectiveness of the erosion control devices shall be made immediately.

#### Maintenance Requirements for inclusion in ESC Narrative

the plan approving authority.

- .. Safety fence shall be checked regularly for weather-related or other damage. Any necessary repairs must be made immediately. 2. Care should be taken to secure all access points (gates) at the end of each working day. All locking devices must be repaired or
- replaced as necessary.
- 1. Silt fences shall be inspected immediately after each rainfall and at least daily during prolonged rainfall. Any required repairs shall be made immediately.
- 2. Close attention shall be paid to the repair of damaged silt fence resulting from end runs and undercutting.
- 3. Should the fabric on a silt fence decompose or become ineffective prior to the end of the expected usable life and the barrier still be necessary, the fabric shall be replaced promptly.
- 4. Sediment deposits should be removed after each storm event. They must be removed when deposits reach approximately one-half the
- 5. Any sediment deposits remaining in place after the silt fence is no longer required shall be dressed to conform with the existing grade, prepared and seeded.

3.07 Storm Drain Inlet Protection . The structure shall be inspected after each rain and repairs made as

- 2. Sediment shall be removed and the trap restored to its original dimensions when the sediment has accumulated to one half the design depth of the trap. Removed sediment shall be deposited in a suitable area and in such a manner that it will not erode.
- 3. Structures shall be removed and the area stabilized when the remaining drainage area has been properly stabilized.

#### 3.09 Temporary Diversion Dike

- .. The measure shall be inspected after every storm and repairs made to the dike, flow channel, outlet or sediment trapping facility, as
- 2. Once every two weeks, whether a storm event has occurred or not, the
- measure shall be inspected and repairs made if needed. 3. Damages caused by construction traffic or other activity must be repaired before the end of each working day

#### **EROSION CONTROL NARRATIVE**

#### PROJECT DESCRIPTION

THE DEVELOPMENT PROPOSES THE CONSTRUCTION OF A 530 LF SHARED-USE ASPHALT TRAIL TO CONNECT PUMP HOUSE DRIVE AND PARK DRIVE. SITE WORK WILL INCLUDE DEMOLITION OF THE EXISTING EARTHEN FOOTPATH AND CLEARING AREA FOR NEW TRAIL. THE 8-FOOT WIDE ASPHALT TRAIL WILL BE CONSTRUCTED ALONG WITH A DRAINAGE SWALE AND BIORETENTION BASIN THAT WILL TREAT STORMWATER AND DISCHARGE TO AN EXISTING STORM STRUCTURE ON-SITE. THERE WILL BE 0.52 AC OF LAND DISTURBANCE WITH THIS PROJECT. THE PROJECT IS ANTICIPATED TO BEGIN IN THE SPRING OF 2019 WITH CONSTRUCTION LASTING APPROXIMATELY ONE MONTH.

#### **EXISTING SITE CONDITIONS**

THE SITE IS LOCATED WITHIN THE RMA OF THE CHESAPEAKE BAY PRESERVATION AREA. THE SITE IS CURRENTLY AN ASPHALT/GRAVEL PARKING LOT USED FOR THE PARK TRAIL SYSTEM THAT WILL REMAIN WITH THIS PROJECT. EXISTING LAND COVER CONSISTS OF HIGHLY COMPACTED GRAVEL / ASPHALT PARKING AREA, SHORT GRASS, AND LIGHTLY WOODED AREAS. EXISTING SLOPES IN THE PARKING LOT AREA RANGE FROM 1 TO 5%, EXISTING SLOPES ALONG THE NORTHEAST OF THE SITE RANGE FROM 20 TO 30%. THE EXISTING SITE RUNOFF INCLUDING SOME OFFSITE RUNOFF FROM THE WOODED LOT TO THE NORTHEAST DOWN THE STEEP SLOPES. THE SITE STORMWATER IS CONVEYED BY A SWALE INTO A DROP INLET STRUCTURE WHICH OUTFALLS SOUTHWEST OF THE SITE INTO THE RPA.

#### **ADJACENT SITE**

THE PROPERTY IS BORDERED TO THE NORTH BY PUMP HOUSE DRIVE, TO THE EAST BY PARK DRIVE, AND TO THE SOUTH AND WEST BY DENSELY WOODED RPA CONTIGUOUS TO THE JAMES RIVER.

#### **OFF-SITE AREAS**

THERE WILL BE MINIMAL WORK WITHIN THE RIGHT-OF-WAY OF PUMP HOUSE DRIVE TO TIE THE PROPOSED TRAIL INTO THE EXISTING CURB RETURN. THERE WILL BE OFFSITE DRAINAGE FROM THE NORTHEAST THAT WILL FLOW THROUGH THE SITE DURING CONSTRUCTION.

#### SOILS

ACCORDING TO THE MOST RECENT DATA AVAILABLE FOR THE CITY OF RICHMOND FROM THE UNITED STATES DEPARTMENT OF AGRICULTURE NATURAL RESOURCES CONSERVATION SERVICE, A SINGLE SOIL TYPE IS REPRESENTED ONSITE. THE SOIL FOUND IS LISTED BELOW:

MAP UNIT: 40 - Udorthents - Dumps Complex, Pits

#### **CRITICAL AREAS**

CRITICAL AREAS INCLUDE EXISTING STEEP SLOPES ALONG THE NORTHEAST OF THE SITE AND PROPOSED DRAINAGE SWALES. ALSO, WORK IN AND AROUND PUBLIC ROADS AND EXISTING PARKING LOT TO REMAIN OPEN DURING CONSTRUCTION.

#### **EROSION AND SEDIMENT CONTROL MEASURES**

UNLESS OTHERWISE INDICATED, ALL VEGETATIVE AND STRUCTURAL EROSION AND SEDIMENT CONTROL PRACTICES SHALL BE CONSTRUCTED AND MAINTAINED ACCORDING TO MINIMUM STANDARDS AND SPECIFICATIONS OF THE VIRGINIA EROSION AND SEDIMENT CONTROL HANDBOOK. THE MINIMUM STANDARDS OF THE VESCH SHALL BE ADHERED TO UNLESS OTHERWISE WAIVED OR APPROVED BY A VARIANCE BY LOCAL **AUTHORITIES HAVING JURISDICTION** 

- 1. INLET PROTECTION (VESCH PRACTICE #3.07-8 AND #3.07-2) IS TO BE PLACED AT THE EXISTING INLET AS INDICATED PRIOR TO THE COMMENCEMENT OF CONSTRUCTION ACTIVITIES. INLET PROTECTION IS TO BE PLACED AT THE PROPOSED BIORETENTION OVERFLOW INLET AS INDICATED AT THE TIME THE STRUCTURE IS SET TO RECEIVE RUNOFF.
- 2. SILT FENCE (VESCH PRACTICE #3.05) WILL BE INSTALLED IN AREAS THAT HAVE POTENTIAL FOR SEDIMENT TO DRAIN OFFSITE
- 3. DIVERSION (VESCH PRACTICE #3.12) WILL BE USED TO CONVEY DRAINAGE TO THE EXISTING STORM INLET. THE DIVERSION WILL BE CONVERTED TO THE PERMANENT DRAINAGE SWALE AS DEPICTED ON THE GRADING PLAN.

#### MANAGEMENT STRATEGIES

- PROVIDE PERMANENT SEEDING OR OTHER STABILIZATION IMMEDIATELY AFTER GRADING.
- 2. ISOLATE TRENCHING FOR UTILITIES AND DRAINAGE FROM DOWNSTREAM

CONVEYANCES IN ORDER TO MINIMUM PERIMETER CONTROLS.

3. ALL EROSION AND SEDIMENT CONTROL PRACTICES SHALL BE MAINTAINED UNTIL THEY ARE NO LONGER REQUIRED TO COMPLY WITH THE CONTRACT DOCUMENTS OR STATE LAW.

#### **VEGETATION PRACTICES**

#### 1. TEMPORARY SEEDING - 3.31

ALL DENUDED AREAS WHICH WILL BE LEFT DORMANT FOR MORE THAN 14 DAYS SHALL BE SEEDED WITH FAST GERMINATING TEMPORARY VEGETATION IMMEDIATELY FOLLOWING GRADING OF THOSE AREAS. SELECTION OF THE SEED MIXTURE SHALL DEPEND ON THE TIME OF YEAR IT IS

2. PERMANENT SEEDING - 3.32

ALL DENUDED ARES SHALL BE SEEDED WITH PERMANENT VEGETATION FOR FINAL STABILIZATION OF THE PROJECT

3. VEG- 3.37

ALL DENUDED AREAS WHERE VEGETATION OTHER THAN TURF IS PREFERRED SHALL BE STABILIZED BY ESTABLISHING VEGETATIVE COVER WITH TREES, SHRUBS, VINES, OR GROUND COVERS. APPLY AS INDICATED (ALSO REFER TO LANDSCAPE PLAN FOR AREAS WHERE ORNAMENTALS, TREES AND SHRUBS ARE PLANNED.)

#### PERMANENT STABILIZATION

SEEDING SHALL BE IN ACCORDANCE WITH STD. & SPEC. 3.32, PERMANENT SEEDING. SEED TYPE SHALL BE AS SPECIFIED FOR "MINIMUM CARE LAWNS" AND "GENERAL SLOPES" IN THE HANDBOOK.

- 1. ANY DISTURBED AREA NOT PAVED, SODDED, OR BUILT UPON, WILL HAVE A VEGETATIVE COVER PRIOR TO FINAL INSPECTION, AND IN THE OPINION OF THE DCR WILL BE MATURE ENOUGH TO CONTROL SOIL EROSION SATISFACTORILY AND SURVIVE SEVER WEATHER CONDITIONS.
- 2. WINTERIZATION-ANY DISTURBANCE AREA NOT PAVED, SODDED, OR BUILT UPON BY OCTOBER 15TH IS TO BE SEEDED AND MULCHED ON THAT DATE UNLESS WAIVED BY THE DCR.
- 3. TEMPORARY SEEDING WILL BE APPLIED WITHIN 7 DAYS TO DENUDED AREAS WHICH MAY NOT BE AT FINAL GRADE BUT WILL REMAIN DORMANT (UNDISTURBED) FOR LONGER THAN 14 DAYS. FOR TEMPORARY SEEDING USE 50% OF THE RECOMMENDED RATES OF FERTILIZER, LIME AND FULL AMOUNT OF SEED AND MULCH REQUIRED FOR REGULAR SEEDING.
- 4. ELECTRIC POWER, TELEPHONE, AND GAS SUPPLY TRENCHES ARE TO BE COMPACTED, SEED AND MULCHED WITHIN 7 DAYS AFTER BACKFILL.
- 5. ALL TEMPORARY EARTH BERMS, DIVERSIONS, AND SILT DAMS ARE TO BE MULCHED AND SEEDED FOR VEGETATIVE COVER IMMEDIATELY AFTER GRADING. STRAW OR HAY MULCH IS REQUIRED. THE SAME APPLIES TO ALL STOCKPILES, ON SITE AS WELL AS SOIL TRANSPORTED FROM THE PROJECT SITE.

#### **CHECK SET NOT FOR CONSTRUCTION**

10/25/2019 DRAWN BY . NORDBERG

**DESIGNED BY** I. NORDBERG CHECKED BY

.A. GUDORP SCALE

N/A

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JOB NO. 6157.002

SHEET NO. C5.2